

Dubai Metro – Project Outline

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Dubai Metro – Project outline

Red & Green lines under construction

- Red Line Opened Sept 2009
- Green Line Opening 2011

Other lines to follow:

- Purple line
 - Airport express
- Blue line
 - Along Emirates Road

Dubai Metro will be:

- Driverless
- Fully-automated
- Longest in the world

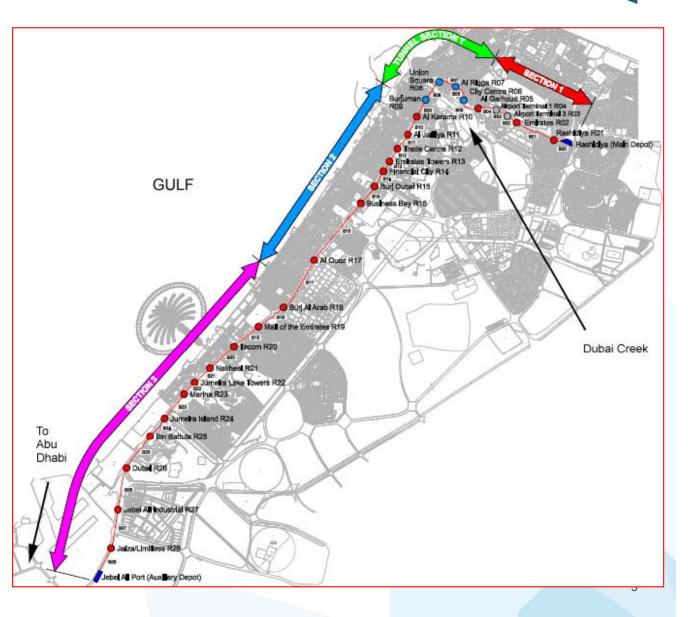


Dubai Metro – Project outline

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Red line

- 52 km route
- 42 km viaduct
- 22 overground stations
- 5.5 km tunnels
- 4 underground stations
- 2 depots



Dubai Metro – Project outline

Green line

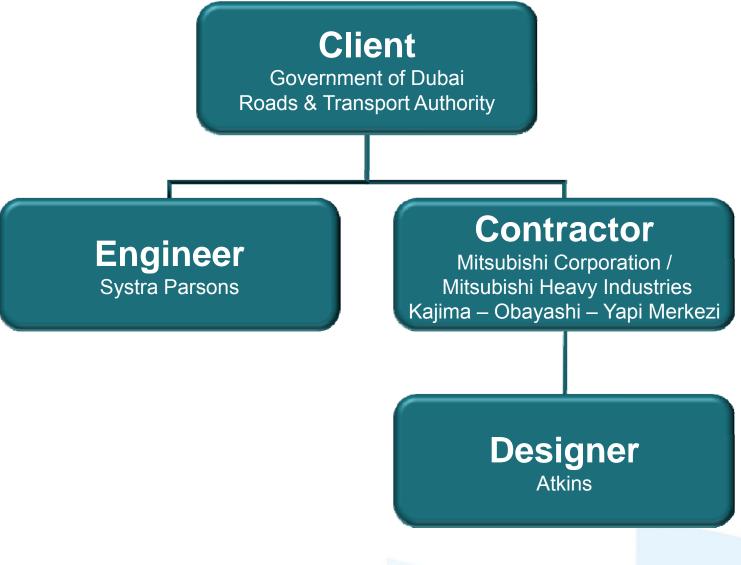
- 24 km route
- 16 km viaduct
- 12 overground stations
- 7.0 km tunnels
- 8 underground stations
- 1 depot



Dubai Metro – Project organisation



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Recent

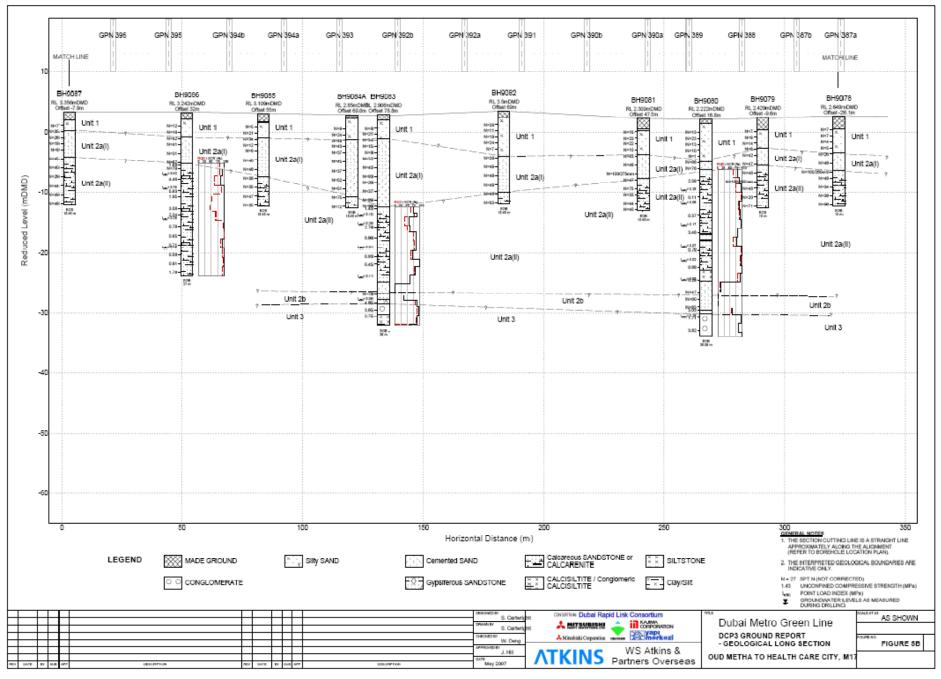
Unit 1: Dune Sands & Sabkha Deposits

Pleistocene (2 million years)

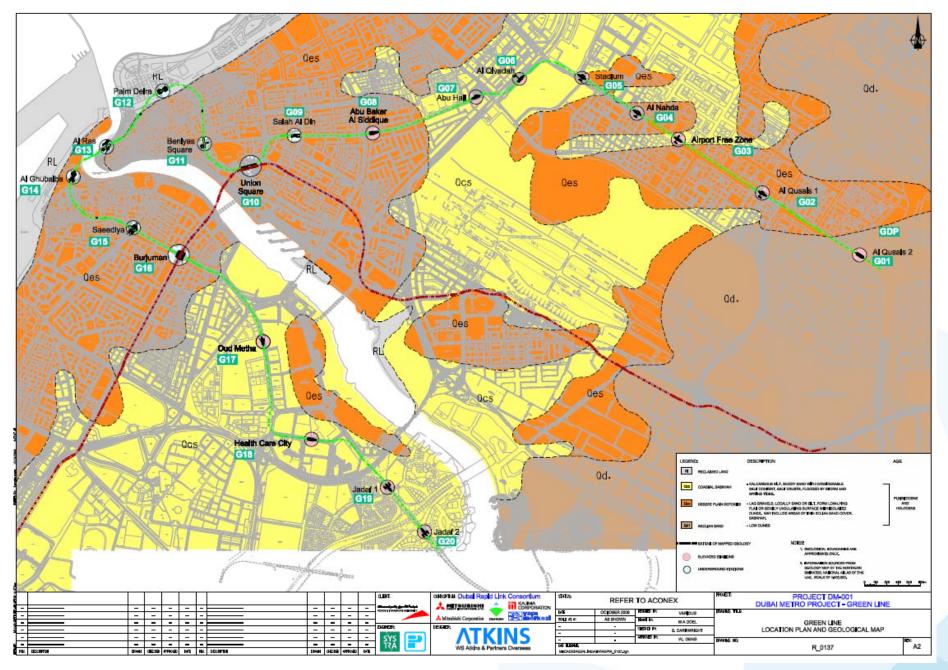
- Unit 2a(i): Marine Sands, weakly cemented
- Unit 2a(ii): Marine Calcarenite, very weak to weak
- Unit 2b: Aeolian Gypsiferous Sandstone, very weak to weak

Mio-Pliocene – Barzaman Formation (20 million years)

Unit 3: Conglomerates, mudstones and siltstones (Wash from Hazar Mountains)



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Dubai Metro – Geological Investigation

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Red Line: Over 1200 boreholes and CPTs Green Line: Over 600 boreholes and CPTs



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Cable Percussion Boring & Cone Penetration Testing

• Sands and Weakly Cemented Sands

Rotary Coring

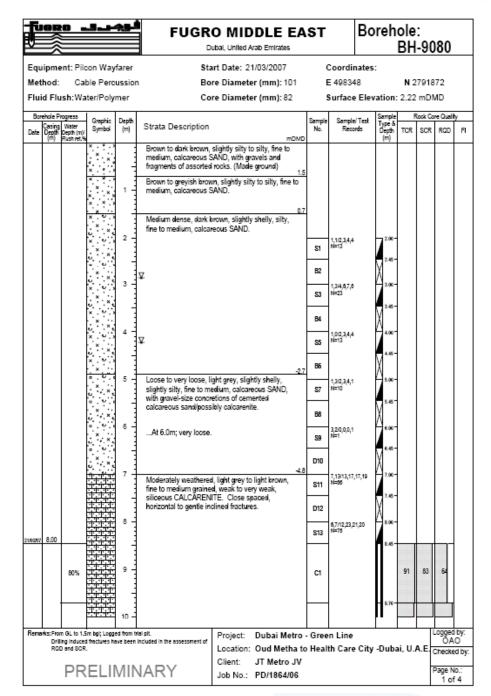
• Sandstone, Calcisilitite, Conglomerate

In situ testing

- SPT
- In situ permeability

Laboratory Testing

- Moisture Content, PSD, Sulphate, pH on soils
- Point Load, Unconfined Compressive Strength on Rock



Unit 2a(i)





Unit 2a(ii)

Unit 2b









Dubai Metro – Derivation of Design Parameters

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Dubai Metro – Derivation of Design Parameters

- Design parameters derived by Atkins Dubai and agreed with the Engineer in Dubai
- Atkins Dubai produce a Ground Report and Pile Length Report
- No opportunity to re-negotiate design parameters
- Design parameters in soil and rock derived from SPT- N value and Unconfined Compressive Strength (UCS) respectively

Dubai Metro – Derivation of Design Parameters

Φ_{soil} from SPT N-value after Peck, Hanson and Thorburn
Φ_{rock} from triaxial test results

•c'_{rock} from c' = UCS_{mass}(1-sin Φ)/2 cos Φ

•E' _{soil} = 2.3N₆₀ MPa •E' _{rock} = 215 x UCS^{0.5}

 $f_{s(soil)} = 1.6N + 6 kPa after Decourt (1995)$ $f_{s(rock)} = 02.5 \text{ or } 0.35 \times (UCS_{design})^{0.5} after Zhang & Einstein (1998)$ $f_{b(rock)} = 2.5 \times (UCS_{design})^{0.5}$

Dubai Metro – Derivation of Design Parameters

Typical Design Parameters

		ν	γ _b (Mg/m³)	c' (kN/m²)	φ (°)	f _s (kN/m²)	E _{sv} ' (kN/m²)	E _{sh} ' (kN/m²)
Soil	Unit 1	0.25	1.9	0	30	14	21	15
	Unit 2a(i)	0.25	1.9	5	36	38	69	48
Rock	Unit 2a (ii)	0.2	2.0	72	39	270	180	126
	Unit 2b	0.2	2.0	60	39	210	167	117
	Unit 3	0.2	2.0	90	35	275	215	151



Dubai Metro – Viaduct Substructure Design

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Dubai Metro – Viaduct Substructure Design

Viaduct arrangement

- Precast segmental construction
 - Single spans of 20m to 36m
 - Twin spans of either 44m+44m or 40m+40m made continuous after deck erection
 - 3-span continuous structures made by balanced cantilever method with main spans of 66m to 74m
 - 3-span or 4-span continuous structures of 30m to 36m through elevated stations



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Dubai Metro – Viaduct Substructure Design **MTKINS**

Pile design

- Single central piers typically 2.2m to 2.8m diameter
- Twin-pile groups, typically 1.6m to 1.8m diameter, used to span over existing services
- 4-pile groups, typically 1.6m to 1.8m diameter, for more heavily loaded internal piers of continuous spans
- Piles up to 40 metres long
- Lengths determined from critical SLS or ULS cases
 - Based on skin friction safety factors of 2.5 and 1.5 respectively
 - End bearing ignored



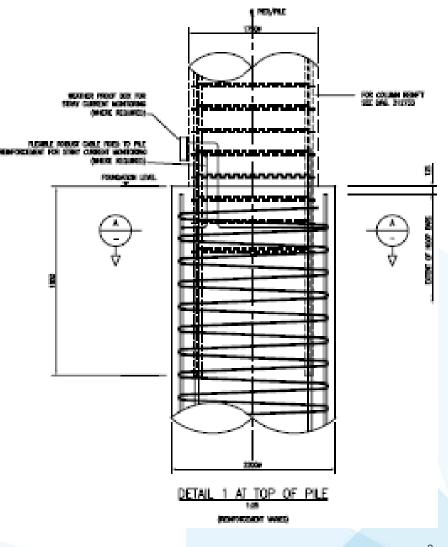
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Dubai Metro – Viaduct Substructure Design

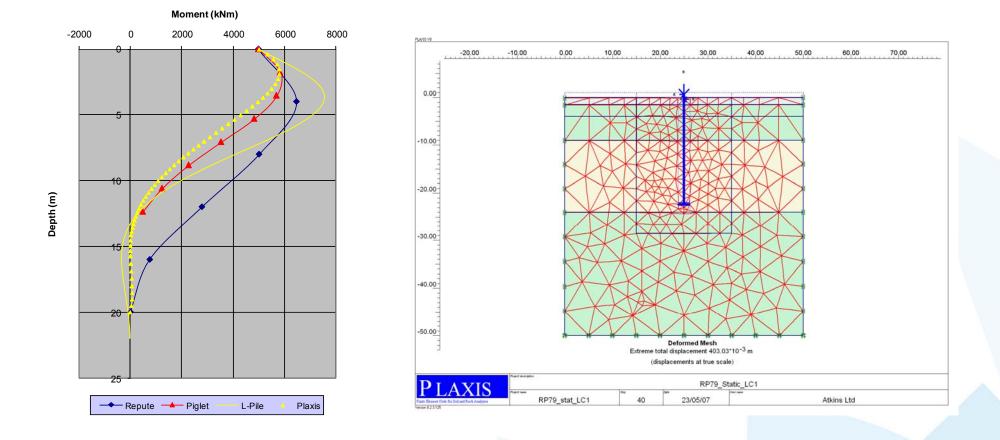
Pile Design (continued)

- Designed for durability in aggressive environment
 - Addition of waterproofing membrane to protect against chloride attack
 - Pile cover of 120mm to protect against sulphate attack
- Horizontal ground acceleration coefficient of 0.12g (ULS only)
- Centrifugal loading (plan curvature down to 300m)
- Self-weight of deck on curved sections of deck
- Wind
- Collision loads
- Rail-structure interaction



Dubai Metro – Viaduct Substructure Design Pile Analysis

 Load effects for reinforcement design generated by analysis using REPUTE program and verified using PLAXIS, PIGLET & L-PILE



MTKINS Dubai Metro – Viaduct Substructure Design

Pile Testing

- 3no piles tested
- Showed 2 x calculated ultimate skin friction and 50% calculated end bearing
- Recommended 0.35 $(UCS_{design})^{0.5}$ for polymer supported pile shaft and 0.25 $(UCS_{design})^{0.5}$ for bentonite supported pile shaft as minimum values
- Recommended $Q_{allow} = Qs + Q_b/2.5$



Dubai Metro – Viaduct Substructure Design

Pile Testing (continued)

Pile No.	Diameter (m)	Length (m)	Design SWL (kN)	Maximum Test Load (kN)	Settlement at SWL (mm)	Settlement at Max Load (mm)
OP1	2.2	20	13,500	54,000	1.4	6.9
OP3	2.2	25	11,000	24,750	2	5
OP4	1.5	20	9,500	38,000	2	12



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- First Underground Station to be constructed
- On-line junction of Red and Green Lines
- Internal dimensions 50m x 250m
- Open aspect with few internal columns (25 metre span with internal columns)
- Drive shaft for all Red and Green Line tunnel drives



Design Considerations

- Wall depth to -44.6mDMD to provide satisfactory FoS against flotation and reduce water ingress during construction (Tender design had grout plug)
- Base slab design to counteract uplift from 22 metres of water, 2.5 metres thick (tension barrettes)
- Wall design for high moments from large spans of slabs, 1200mm thick to optimise reinforcement density (reverse moment from uplift of base slab)
- Wall design to BS8110 and BS8007 (Reinforcement controlled by 0.2mm crack width requirement)



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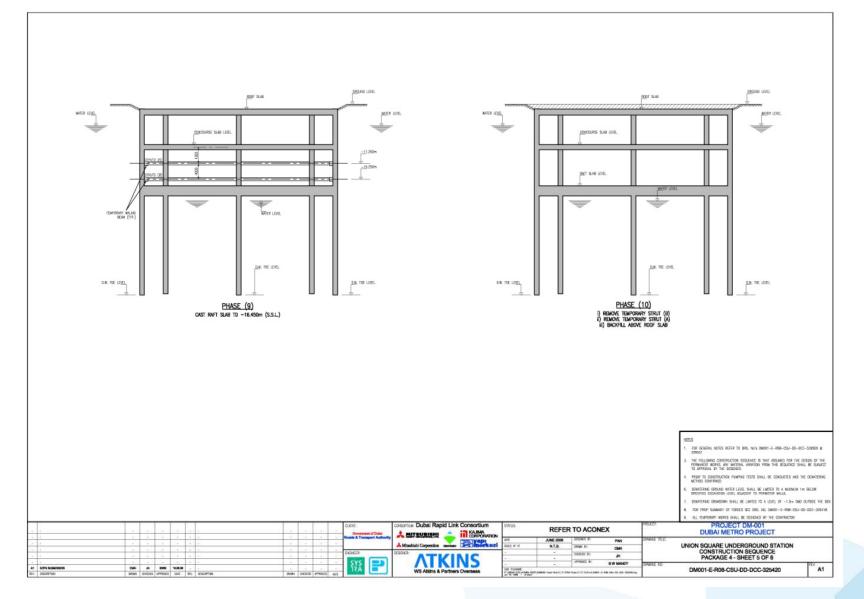
Panel Arrangement – Eastern End of Station Box

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A6

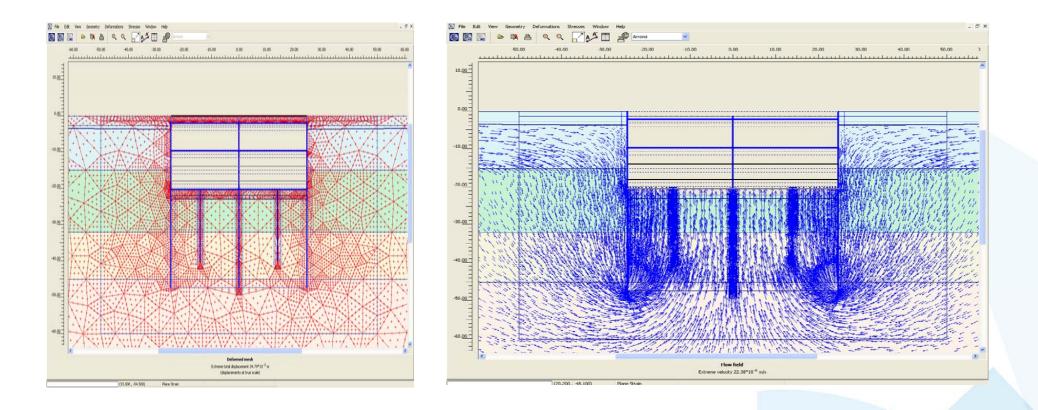
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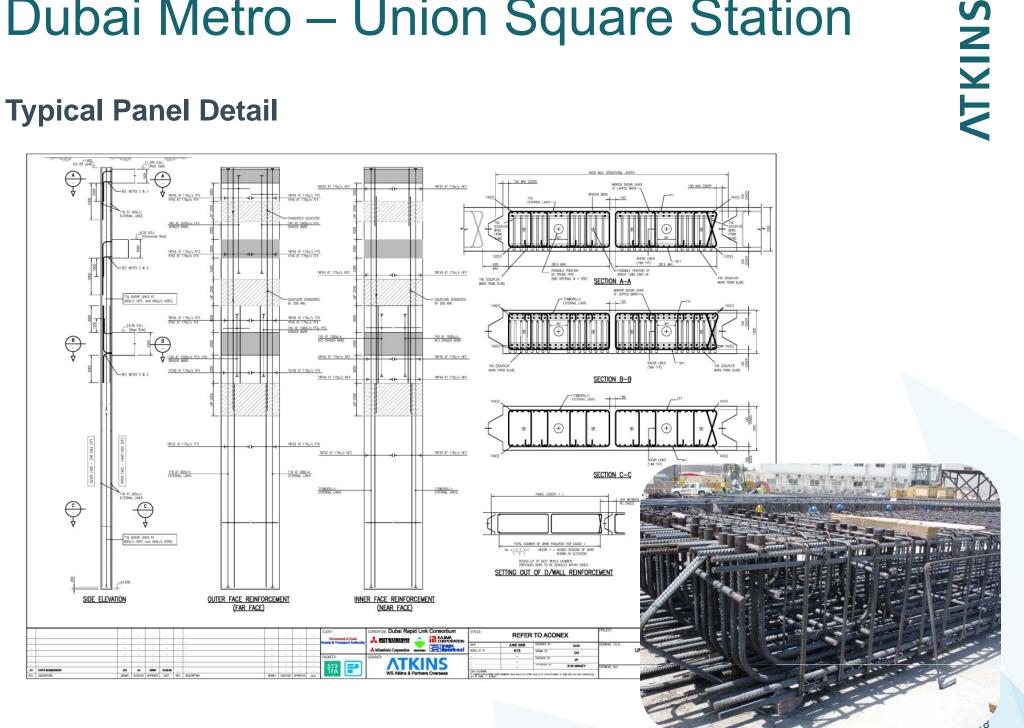
Typical sections through box



Design analysis

- Wall reinforcement design after CIRIA C580 using PLAXIS
- Flotation design using I. Struct. E. Guidelines for Basement design
- Settlement Analysis using approach after CIRIA C580







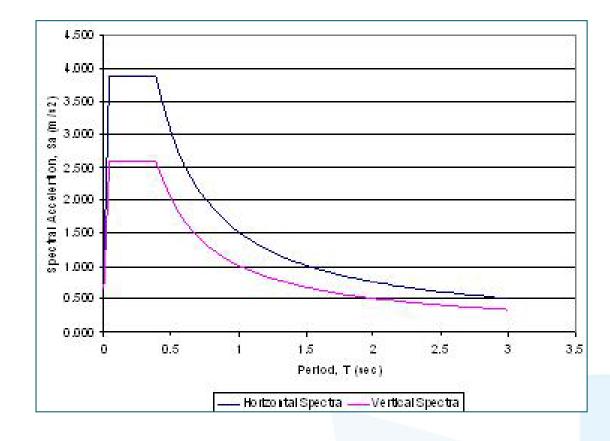
Dubai Metro – Questions

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Dubai Metro – Substructure design

Seismic analysis

- Nearest fault line is 120 km from UAE Zargos fault line
- Cautious approach because of use for evacuation "essential" to AASHTO
- AASHTO defined seismic response spectrum used with A = 0.12g
 - Site coefficient dependent on bearing type used



Dubai Metro – Substructure design

Column and pile design

- Once rules established, 1400+ unique foundations designed in 9 months
- Strict control procedures between design team & setting-out team
- Optimisation / automation process developed throughout
 - Process of seismic analysis and section checking automated by linked macros
 - Enabled peak output of 100 foundations to be designed per week



